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WIND LOAD COMPONENT AND CLADDING
WALL PRESSURE DIAGRAM

WIND LOAD COMPONENT AND CLADDING
ROOF PRESSURE DIAGRAM

BUILDING DATA

ASCE 1-10 WIND

WIND VELOCITY u w

150 mph

ATMOSPHERIC PRESSURE

14.7 lb/ft²

WIND VELOCITY u w

140 mph

(CELORED ENVELOPMENT 1-70)

WIND PRESSURE

HEAD pressure assumes component, course

ROOF DECK LOAD REGISTRATION (psf)

6.7 sf

WIND ANGLE (° DEGREES)

7.27 degrees

WIND EXPOSURE CATEGORY

C

REGULATORY REQUIREMENTS

PERIODIC PRESSURE TESTING OF WIND-RESISTANT WALLS.

WIND LOADS - MEMBERS, SHELVES, DEVICES AND CLADDING

DESIGN WIND LOADS - METHODS 1 & 2 - FLA 1215 - CL3

NOTE: Pressures shown shall be multiplied by its appropriate load factor from ASCE 7.10

REV: BY DATE DESCRIPTION
GENERAL NOTES

1. THIS STRUCTURE WAS DESIGNED IN ACCORDANCE WITH THE 2015 INTERNATIONAL BUILDING CODE, BUILDING (I.B.C.)
2. ALL MATERIALS AND LABOR SHALL BE IN ACCORDANCE WITH THE ABOVE CODE AND ALL OTHER APPLICABLE LOCAL CODES AT THE TIME OF MANUFACTURE.
3. WRITTEN DIMENSIONS SHALL TAKE PRECEDENCE OVER SCALLED DIMENSIONS.
4. THE FOUNDATION PLAN IS A SEPARATE SET OF PLANS FOR APPROVAL BY LOCAL MUNICIPALITIES.
5. EXTERIOR DIMENSIONS CAN VARY BETWEEN LIMITS SHOWN AT 2' O.C. BUT MEMBER SPACING SHALL NOT EXCEED LIMITS AS INDICATED.
6. ALL THE FOLLOWING TIMBER SHALL BE PRESSURE TREATED IN ACCORDANCE WITH APA USE CATEGORY GC4B (GROUND CONTACT, HEAVY DUTY). SKIDS
7. ALL THE FOLLOWING TIMBER SHALL BE PRESSURE TREATED IN ACCORDANCE WITH APA USE CATEGORY GC3B (EXTERIOR ABOVE GROUND, UNCOATED OR POOR WATER RUN-OFF). FLOOR JOISTS, PLYWOOD FLOOR DECKING, AND EXTERIOR RATED WOOD STRUCTURAL PANEL SIDING.
8. ALL FASTENERS AND CONNECTORS IN CONTACT WITH PRESSURE TREATED WOOD SHALL BE HOT DIPPED GALVANIZED (6185) OR STAINLESS STEEL.
9. ALL WINDOWS WITHIN 24" OF DOORS, AND ALL GLASS IN DOORS SHALL BE SAFETY, TEMPERED, OR ACRYLIC PLASTIC SHEET.
10. FOR ROOFS WITH ASPHALT SHINGLES AND A SLOPE BETWEEN 2 TO 12 AND 4 TO 12 SHALL HAVE A DOUBLE UNDERLAMINATION AS REQUIRED IN ACCORDANCE WITH SECTION 1501.2.8 OF THE 2015 I.B.C.
11. UNDERLAMINATION SHALL CONFORM WITH SECTION 1501.2.3 OF THE 2015 I.B.C.
12. ASPHALT SHINGLES SHALL CONFORM WITH SECTION 1501.2.5 OF THE 2015 I.B.C. ATTACHMENT OF ASPHALT SHINGLES SHALL CONFORM WITH 1501.2.7 OF THE 2015 I.B.C.
13. FASTENERS FOR ASPHALT SHINGLES SHALL CONFORM TO SECTION 1501.2.6 OF THE 2015 I.B.C.
14. TIE-DOWNS SHALL MEET THE REQUIREMENTS OF ALL APPLICABLE CODES.
15. THESE PLANS HAVE NOT BEEN DESIGNED FOR MVHZ REQUIREMENTS AS SET FORTH IN THE 2015 I.B.C. OR FOR USE AS A COMMERCIAL BUILDING.
16. THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING ALL EXISTING CONDITIONS.
17. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DELIVERY AND PLACEMENT OF LAWN STORAGE UNIT TO ENSURE THE INTEGRITY OF THE BUILDING AND ITS COMPONENT PARTS.
18. NO FIELD REVISIONS TO ANY STRUCTURAL COMPONENTS OR DEVIATIONS FROM THESE DRAWINGS SHALL BE MADE.
19. THE OWNER AND THE CONTRACTOR SHALL, HOLD HARMLESS THE ENGINEER FROM AND AGAINST ALL LIABILITY CLAIMS, DAMAGES, LOSSES AND EXPENSES INCLUDING LEGAL FEES ARISING OUT OF OR RESULTING FROM ERRORS OR OMISSIONS IN THE PERFORMANCE OF THE WORK BY THE CONTRACTOR.
20. SECTIONS AND DETAILS ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL APPLY AT ALL SIMILAR LOCATIONS, UNLESS OTHER SECTIONS AND DETAILS ARE SPECIFICALLY REFERENCED.
21. REFER TO SUPPLIED FASTENING SCHEDULE FOR FASTENING BASED ON CONNECTION AND LOCATION OF MEMBERS AS PER 2015 INTERNATIONAL BUILDING CODE TABLE 2304.1 UNLESS NOTED OTHERWISE.
22. BUILDINGS HAVE BEEN DESIGNED FOR LP SMARTSIDE PRECISION PANEL SIDING, LP SMARTSIDE PRECISION LAM SIDING SHALL NOT BE USED.
23. FASTENERS IN LP SMARTSIDE PRECISION PANEL SIDING MUST NOT BE INSTALLED IN PANEL SIDING GROOVES IN THE FIELD OF PANEL SIDING OR WHEN THE PANEL SIDING GROOVES OCCUR AT CUT EDGES OF THE PANEL SIDING.
24. REFER TO THE I-CES EVALUATION REPORT ESR-1301 FOR ADDITIONAL DATA AND SPECIFICATIONS OF LP SMARTSIDE PRECISION PANEL SIDING.
25. MAX OPENINGS WIDTHS MUST COMPLY WITH DESIGN RATIOS AS PER ANS/AFIA E15.2-2005. BUILDINGS HAVE BEEN DESIGNED TO HAVE ONLY OPENINGS WITH MAX WIDTHS EQUAL TO THOSE IN THE ENDWALL SHEAR CHART.
26. PER SECTION 1601.1.2 #3 OF THE FBC, STORAGE SHEDS THAT ARE NOT DESIGNED FOR HUMAN HABITATION AND THAT HAVE A FLOOR AREA OF 120 SQUARE FEET OR LESS ARE NOT REQUIRED TO COMPLY WITH THE MANDATORY HIND-BORNE-DEBRIS-I M PACT STANDARDS OF THE INTERNATIONAL BUILDING CODE.
27. BUILDINGS THAT ARE 400 SQUARE FEET OR LESS AND THAT ARE INTENDED FOR USE IN CONJUNCTION WITH ONE-AND-TWO- FAMILY RESIDENCES ARE NOT SUBJECT TO THE DOOR HEIGHT AND WIDTH REQUIREMENTS OF THE MANDATORY HIND-BORNE-DEBRIS-I M PACT STANDARDS OF THE INTERNATIONAL BUILDING CODE.
28. BUILDINGS HAVE BEEN DESIGNED TO HAVE ANCHORS DIRECTLY ATTACHED TO ALL FOUR CORNERS OF THE BUILDING TO RESIST TENSION FORCES FROM LATERAL WIND LOADS. THIS DESIGN CONSIDERATION MUST BE MADE BY INSTALLER WHEN ATTACHING ANCHORING SYSTEM TO BUILDING.
29. UNLESS NOTED OTHERWISE, ATTACH ALL MANUFACTURED PRODUCTS IN ACCORDANCE WITH MANUFACTURERS SPECIFICATIONS.

SITE INSTALLED ITEMS

NOTE THAT THIS LIST DOES NOT NECESSARILY LIMIT THE ITEMS OF WORK AND MATERIALS THAT MAY BE REQUIRED FOR A COMPLETE INSTALLATION. ALL SITE RELATED ITEMS ARE SUBJECT TO LOCAL JURISDICTIONAL APPROVAL.

1. THE COMPLETE FOUNDATION SUPPORT AND TIE-DOWN SYSTEM.
2. RAMPS, STAIRS, AND GENERAL ACCESS TO THE BUILDING IF NECESSARY.
3. GUTTERS AND DRAIN SPOTS ON ALL BUILDINGS WITH SAVES OF LESS THAN 6 INCHES HORIZONTAL PROJECTION EXCEPT FOR GABLE END RAKES.

ASSUMPTIONS

1. THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE 2015 INTERNATIONAL BUILDING CODE, BUILDING (I.B.C.)
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29. UNLESS NOTED OTHERWISE, ATTACH ALL MANUFACTURED PRODUCTS IN ACCORDANCE WITH MANUFACTURERS SPECIFICATIONS.

OCCUPANCY NOTE:
 THIS BUILDING IS NOT DESIGNED FOR HUMAN HABITATION AND DOES NOT HAVE RUNNING WATER OR SANITATION SERVICES. THIS BUILDING IS DESIGNED AS A UTILITY SHED TO STORE LAWN EQUIPMENT SUCH AS WHEEL BARROWS, GARDENING SUPPLIES, FLOWER POTS, AND CARDBOARD BOXES WITH VARIOUS SMALL ITEMS.
### Fastening Schedule

<table>
<thead>
<tr>
<th>Connection</th>
<th>Fasteners &amp; Type</th>
<th>Location</th>
</tr>
</thead>
<tbody>
<tr>
<td>1. Joist to sill or girder</td>
<td>3 - 8d common (2 1/2&quot; x 0.13&quot;)</td>
<td>toenail</td>
</tr>
<tr>
<td></td>
<td>3 - 3&quot; x 0.131&quot; nails</td>
<td></td>
</tr>
<tr>
<td></td>
<td>3 - 3&quot; 14 gauge staples</td>
<td></td>
</tr>
<tr>
<td>2. Bridging to joist</td>
<td>2 - 8d common (2 1/2&quot; x 0.13&quot;)</td>
<td>toenail each end</td>
</tr>
<tr>
<td></td>
<td>2 - 3&quot; x 0.131&quot; nails</td>
<td></td>
</tr>
<tr>
<td></td>
<td>2 - 3&quot; 14 gauge staples</td>
<td></td>
</tr>
<tr>
<td>3. Sole plate to joist or blocking</td>
<td>16d common (3 1/2&quot; x 0.185&quot;) at 16&quot; o.c.</td>
<td>typical face nail</td>
</tr>
<tr>
<td></td>
<td>3&quot; x 0.131&quot; nails at 8&quot; o.c.</td>
<td></td>
</tr>
<tr>
<td></td>
<td>3&quot; 14 gauge staples at 12&quot; o.c.</td>
<td></td>
</tr>
<tr>
<td>4. Sole plate to joist or blocking at braced wall panel</td>
<td>3 - 8d common (3 1/2&quot; x 0.185&quot;) at 16&quot; o.c.</td>
<td>braced wall panels</td>
</tr>
<tr>
<td></td>
<td>3 - 3&quot; x 0.131&quot; nails at 8&quot; o.c.</td>
<td></td>
</tr>
<tr>
<td></td>
<td>4 - 3&quot; 14 gauge staples at 12&quot; o.c.</td>
<td></td>
</tr>
<tr>
<td>5. Top plate to stud</td>
<td>2 - 8d common (3 1/2&quot; x 0.185&quot;)</td>
<td>end nail</td>
</tr>
<tr>
<td></td>
<td>2 - 3&quot; x 0.131&quot; nails</td>
<td></td>
</tr>
<tr>
<td></td>
<td>2 - 3&quot; 14 gauge staples</td>
<td></td>
</tr>
<tr>
<td>6. Stud to sole plate</td>
<td>4 - 8d common (2 1/2&quot; x 0.13&quot;)</td>
<td>toenail</td>
</tr>
<tr>
<td></td>
<td>4 - 3&quot; x 0.131&quot; nails</td>
<td></td>
</tr>
<tr>
<td></td>
<td>4 - 3&quot; 14 gauge staples</td>
<td></td>
</tr>
<tr>
<td></td>
<td>2 - 8d common (3 1/2&quot; x 0.185&quot;)</td>
<td>end nail</td>
</tr>
<tr>
<td></td>
<td>2 - 3&quot; x 0.131&quot; nails</td>
<td></td>
</tr>
<tr>
<td></td>
<td>2 - 3&quot; 14 gauge staples</td>
<td></td>
</tr>
<tr>
<td>7. Double studs</td>
<td>16d common (3 1/2&quot; x 0.185&quot;) at 24&quot; o.c.</td>
<td>face nail</td>
</tr>
<tr>
<td></td>
<td>3&quot; x 0.131&quot; nails at 8&quot; o.c.</td>
<td></td>
</tr>
<tr>
<td></td>
<td>3&quot; 14 gauge staples at 12&quot; o.c.</td>
<td></td>
</tr>
<tr>
<td>8. Double to plates</td>
<td>16d common (3 1/2&quot; x 0.185&quot;) at 16&quot; o.c.</td>
<td>typical face nail</td>
</tr>
<tr>
<td></td>
<td>3&quot; x 0.131&quot; nails at 12&quot; o.c.</td>
<td></td>
</tr>
<tr>
<td></td>
<td>3&quot; 14 gauge staples at 12&quot; o.c.</td>
<td></td>
</tr>
<tr>
<td></td>
<td>16d common (3 1/2&quot; x 0.185&quot;) at 16&quot; o.c.</td>
<td>lap splice</td>
</tr>
<tr>
<td></td>
<td>2 - 3&quot; x 0.131&quot; nails</td>
<td></td>
</tr>
<tr>
<td></td>
<td>2 - 3&quot; 14 gauge staples</td>
<td></td>
</tr>
<tr>
<td>9. Blocking between joists or rafters to top plate</td>
<td>3 - 8d common (2 1/2&quot; x 0.13&quot;)</td>
<td>toenail</td>
</tr>
<tr>
<td></td>
<td>3 - 3&quot; x 0.131&quot; nails</td>
<td></td>
</tr>
<tr>
<td></td>
<td>3 - 3&quot; 14 gauge staples</td>
<td></td>
</tr>
<tr>
<td>10. Rim joist to top plate</td>
<td>8d common (2 1/2&quot; x 0.13&quot;) at 8&quot; o.c.</td>
<td>toenail</td>
</tr>
<tr>
<td></td>
<td>3&quot; x 0.131&quot; nails at 6&quot; o.c.</td>
<td></td>
</tr>
<tr>
<td></td>
<td>3&quot; 14 gauge staples at 6&quot; o.c.</td>
<td></td>
</tr>
<tr>
<td>11. Top plates, laps and intersections</td>
<td>2 - 8d common (3 1/2&quot; x 0.185&quot;)</td>
<td>face nail</td>
</tr>
<tr>
<td></td>
<td>2 - 3&quot; x 0.131&quot; nails</td>
<td></td>
</tr>
<tr>
<td></td>
<td>2 - 3&quot; 14 gauge staples</td>
<td></td>
</tr>
<tr>
<td>12. Continuous header (2) pieces</td>
<td>16d common (3 1/2&quot; x 0.185&quot;)</td>
<td>16&quot; O.C. along edge</td>
</tr>
</tbody>
</table>

(continued)
<table>
<thead>
<tr>
<th>CONNECTION</th>
<th>FASTENING</th>
<th>LOCATION</th>
</tr>
</thead>
<tbody>
<tr>
<td>13. Ceiling Joists to Plate</td>
<td>3 - 8d Common (2 1/2&quot; x 0.131&quot;) 5 - 3&quot; x 0.131&quot; Nails 5 - 3&quot; 14 gage staples</td>
<td>Toenail</td>
</tr>
<tr>
<td>14. Continuous Header to Stud</td>
<td>4 - 8d Common (2 1/2&quot; x 0.131&quot;)</td>
<td>Toenail</td>
</tr>
<tr>
<td>15. Rafter to Plate</td>
<td>3 - 8d Common (2 1/2&quot; x 0.131&quot;) 3 - 3&quot; x 0.131&quot; Nails 3 - 3&quot; 14 gage staples</td>
<td>Toenail</td>
</tr>
<tr>
<td>16. 1&quot; Diagonal Brace to Each Stud and Plate</td>
<td>2 - 8d Common (2 1/2&quot; x 0.131&quot;) 2 - 3&quot; x 0.131&quot; Nails 3 - 3&quot; 14 gage staples</td>
<td>Face Nail</td>
</tr>
<tr>
<td>17. Built-Up Corner Studs</td>
<td>16d (3 1/2&quot; x 0.188&quot;) 3&quot; x 0.131&quot; Nails 3&quot; 14 gage staples</td>
<td>24&quot; O.C. 18&quot; O.C. 16&quot; O.C.</td>
</tr>
<tr>
<td>18A. Built-Up Girder and Beams</td>
<td>20d Common (4&quot; x 0.142&quot;) 32&quot; O.C. 3&quot; x 0.131&quot; Nail at 24&quot; O.G. 3&quot; 14 gage staple at 24&quot; O.C. 2 - 20d Common (4&quot; x 0.142&quot;) 3&quot; x 0.131&quot; Nails 3 - 3&quot; 14 gage staple</td>
<td>Face Nail at top and bottom staggered on opposite sides Face Nail at ends and at each splice</td>
</tr>
<tr>
<td>19. Collar Tie to Rafter</td>
<td>3 - 10d Common (3&quot; x 0.148&quot;) 4 - 3&quot; x 0.125&quot; Nails 4 - 3&quot; 14 gage staples</td>
<td>Face Nail</td>
</tr>
<tr>
<td>20. Roof Rafter to 2-by Ridge Beam</td>
<td>2 - 16d Common (3 1/2&quot; x 0.162&quot;) 3 - 3&quot; x 0.131&quot; Nails 3 - 3&quot; 14 gage staples</td>
<td>Toenail or Face Nail</td>
</tr>
<tr>
<td>21. Joist to Band Joist</td>
<td>3 - 8d Common (3 1/2&quot; x 0.162&quot;) 4 - 3&quot; x 0.131&quot; Nails 4 - 3&quot; 14 gage staples</td>
<td>Face Nail</td>
</tr>
</tbody>
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<tbody>
<tr>
<td>22. Wood Structural Panels and Particleboard Subfloor, Roof and Wall Sheathing (to Framing)</td>
<td>1/2&quot; and Less</td>
<td>6d-1</td>
</tr>
<tr>
<td></td>
<td>1/32&quot; to 1/16&quot;</td>
<td>2 3/8&quot; x 0.13&quot; Nail</td>
</tr>
<tr>
<td></td>
<td>1 3/4&quot; to 1 1/8&quot;</td>
<td>4d - 2 1/2&quot;</td>
</tr>
<tr>
<td></td>
<td>6d COMMON (ROOF SHEATHING)</td>
<td>2 3/8&quot; x 0.13&quot; Nail</td>
</tr>
<tr>
<td></td>
<td>1 3/4&quot; to 1 1/2&quot;</td>
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<td>4d - 2 1/2&quot;</td>
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</tbody>
</table>
FLOOR FRAMING PLAN

1. 1/2" WIDE UNIT WITH 5 SKIDS

FLOOR DECK PLAN

2. SCALE 1/4" = 1'-0"

ROOF FRAMING PLAN

3. SCALE 1/4" = 1'-0"

4. SCALE 1/4" = 1'-0"

THESE PLANS WERE DESIGNED IN ACCORDANCE WITH THE 2015 INTERNATIONAL BUILDING CODE, BUILDING FOR 100 YEAR, SECOND SUST OF 160 MPH.

THOMAS A. DIXON, P.E.
### SHEARWALL CHART

<table>
<thead>
<tr>
<th>BUILDING WIDTH</th>
<th>OPENING WIDTHS IN ENDWALL</th>
<th>MAX LENGTH OF BUILDING</th>
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<tr>
<td></td>
<td>19/32&quot; TI-1&quot;</td>
<td>19/32&quot; LP SMARTPANEL</td>
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<tr>
<td>T-11 1/2&quot;</td>
<td>NONE</td>
<td>23'-4&quot;</td>
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<td>3'-0&quot; MAX</td>
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<tr>
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<td>4'-0&quot;</td>
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<tr>
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<td>24'-9&quot;</td>
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<td>7'-0&quot;</td>
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</tr>
<tr>
<td></td>
<td>NONE</td>
<td>32'-0&quot;</td>
</tr>
<tr>
<td></td>
<td>4'-0&quot; MAX</td>
<td>32'-0&quot;</td>
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<td></td>
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<td></td>
</tr>
<tr>
<td></td>
<td>8'-0&quot;</td>
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**NOTES:**
1. 19/32" TI-1" SHALL BE FASTENED USING 8D COMMON OR DEFORMED NAILS AT 6" O.C. IN FIELD AND 3" O.C. ALONG ALL PANEL EDGES.
2. 19/32" LP SMARTPANEL SHALL BE FASTENED USING 8D COMMON OR DEFORMED NAILS AT 6" O.C. IN FIELD AND 3" O.C. ALONG ALL PANEL EDGES.
3. WINDOWS AND DOORS MAY BE LOCATED IN EITHER THE SIDE WALL OR ENDWALL. DOORS ARE PERMITTED TO BE IN BOTH ENDWALLS OR ENDWALL AND SIDE WALL, IF REQUESTED BY CUSTOMER. LIMITATIONS ON THE TOTAL OPENING DIMENSIONS SHALL BE BASED ON THE SHEAR WALL HEIGHT TO WIDTH RATIO OF 3.5:1 AND SHALL NOT EXCEED (2/3) OF TOTAL LENGTH OF BUILDING. NAILING IN SIDEWALL USE 8D NAILS COMMON OR DEFORMED AT 6" O.C. EVERYWHERE WHEN TOTAL OPENING WIDTHS IN SIDE WALL ARE LESS THAN (2/3) OF TOTAL LENGTH OF BUILDING.

---

**THOMAS A. DIXON, P.E.**

**COOK PORTABLE WAREHOUSES**

**SHEAR WALL TABLE**

**DATE:** 3/30/11

**DRAWN BY:** CNO

**CHECKED BY:** TAD

**SCALE:** AS NOTED

**N.O. NO.:** 445-076
EXAMPLE DRAWING IS OF A 20' LENGTH UNIT

EXAMPLE DRAWING IS OF A 11' KIDE UNIT IV 3'-0" DOOR

EXAMPLE DRAWING IS OF A 11' KIDE UNIT IV 7'-0" DOOR

LOFTED BARN
SIDE ELEVATION
A-5
SCALE 1/4" = 1'-0"

LOFTED BARN
ENDWALL ELEVATION
A-9
SCALE 1/4" = 1'-0"

DESCRIPTION

THOMAS A. DIXON, P.E.

COCO PORTABLE WAREHOUSES
STANDARD BARN SHED
100 DOUGLAS STREET
VALDOSTA, GA 31601
PHONE: 1-229-241-4800

COG OPENINGS

SCS 2013
NFQ AE2314
KSA 21425
SA 2154
NC 33463

WEATHER SANY 3455
HARRISON STREET
THOMASVILLE, GA 31725
PHONE: 1-229-241-9436

EXTERIOR ELEVATIONS

COG PORTABLE WAREHOUSES
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PHONE: 1-229-241-9436

EXTERIOR ELEVATIONS
NOTE:
1. FOR ALL NOTES NOT SHOWN SEE SHEET A-1 - A-3

T-11/2" SHED ENDMALL
WITH NO OPENING
FRAMING ELEVATION
A-4
SCALE 1/4" = 1'-0"

T-11/2" SHED ENDMALL
WITH 4' DOOR OPENING
FRAMING ELEVATION
A-4
SCALE 1/4" = 1'-0"

T-11/2" SHED ENDMALL
WITH 3' DOOR OPENING
FRAMING ELEVATION
A-4
SCALE 1/4" = 1'-0"

PRE-ENGINEERED TRUSS
2x4 S.P.F. #2 TOP PLATE
2x6 S.P.F. #2 TOP PLATE
2x4 S.P.F. #2 VERT. WALL STUD
2'-0' O.C.
2x4 S.P.F. #2 SOLE PLATE

SEE HEADER DETAIL 3/A-11

NOTE:
1. FOR ALL NOTES NOT SHOWN SEE SHEET A-1 - A-3
NOTE:
1. FOR ALL NOTES NOT SHOWN SEE SHEET A-1 - A-4

THESE PLANS WERE DESIGNED IN ACCORDANCE WITH THE 2015 INTERNATIONAL BUILDING CODE, BUILDING CATEGORIES AND SECOND STREET OF 160 MPH.

THOMAS A. DIXON, P.E.

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STANDARD BARN SHED
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VALDOSTA, GA 31601
PHONE: 1-221-241-8805

SCALE: AS NOTED

REV: BY DATE DESCRIPTION

THOMAS A. DIXON, P.E.

COCO PORTABLE WAREHOUSES
STANDARD BARN SHED
100 DOUGLAS STREET
VALDOSTA, GA 31601
PHONE: 1-221-241-8805

SCALE: AS NOTED

REV: BY DATE DESCRIPTION
NOTE:
FOR ALL FASTENING OF FRAMING MEMBERS NOT NOTED ON THIS SHEET REFER TO FASTENING SCHEDULE ON SHEETS C-4 THRU C-6

NOTE:
FOR ALL FASTENING OF FRAMING MEMBERS NOT NOTED ON THIS SHEET REFER TO FASTENING SCHEDULE ON SHEETS C-4 THRU C-6

EXAMPLE DRAWING IS OF A 11'-0" WIDE UNIT
These plans were designed in accordance with the 2015 International Building Code, building type B, second story of 160 MPH.
4x6 P.T.S.Y.P. #2 SKID WITH 1-1/2" WIDE BY 3/4" OR 1-1/2" DEEP NOTCHES AT 16" O.C. ENTIRE LENGTH OF SKID TO ACCOMMODATE FLOOR JOISTS (TYP.)

1/4" x 4" x 30" STEEL SPLICE PLATE ON BOTH SIDES OF SKIDS WITH (2) 5/16" x 2-1/2" LAG SCREWS 3 EACH END OF PLATE

FOR END NOTCH SEE DETAIL #***

NOTE:
FOR ALL FASTENING OF FRAMING MEMBERS NOT NOTED ON THIS SHEET REFER TO FASTENING SCHEDULE ON SHEETS C-4 THRU C-6

THOMAS A. DIXON, P.E.

NO. 12657

AREA FOR APPROVAL STAMPS

COOK PORTABLE WAREHOUSES

THOMAS A. DIXON, P.E.

SEAL OF PROFESSIONAL ENGINEER

SCALE: AS NOTED

NO. 445-016

15 OF 26
**2x4 FLOOR JOIST**

- **2x4 or 2x6 Bottom Chord**
- **1/2" OSB or Plywood Sheathing Fastened with (1) Each 0.131" GUNNAIL or Equal into Rafters and Stud Walls**
- **2x4 Band Joist**
- **2x4 Floor Joist**

**2x4 Stud to Rafter Fastenings in Metal Roof**

- **1/4"-1/2" Ti-Ii Siding or 1/4"-3/8" LP Smartside Precision Panel Siding. See Shear Wall Table 1A-2 for Maximum Building Lengths Based on Prescribed Nailing Pattern in Endwall Shear Walls. Nailing for Sidewalls Shall Be 0.131" GUNNAIL or Equal into Rafters and Stud Walls.**

**2x6 Stud to Rafter Fastenings in Metal Roof**

- **0.148" x 1-1/2" T-GUNNAILED RAFTER TO TOP PLATE**
- **2x6 S.P.F. #2 Top Plate Notched to Receive Siding**
- **1/2" OSB or Plywood Sheathing Fastened with 0.131" GUNNAIL or Equal into Rafters and Stud Walls**

**2x6 or 2x8 S.Y.P. #2 Top Plate Notched to Receive Siding**

- **1/16" OSB or Plywood Sheathing Fastened with 0.131" GUNNAIL or Equal into Top Plate and Stud Walls**

**20 GAUGE x 1-1/4" GALV. STEEL STRAP WITH (20): 0.131"x2-1/4" NAILS OR EQUAL (1) INTO STUD WALLS**

**T-6/8 OSB or Plywood Sheathing Fastened with 0.131" GUNNAIL or Equal at 6" O.C. in Field and Edges**

**2x6 A. PANEL-LOC PLUS ROOF PANEL by CENTRAL STATES MANUFACTURING or EQUAL**

**20465 E. 11-1/2" x 1-1/2" Ti-Ii SIDING or 20465 E. 1-1/2" x 1-1/2" Ti-Ii SIDING**

**NOTE: FOR ALL FASTENING OF FRAMING MEMBERS NOT NOTED ON THIS SHEET REFER TO FASTENING SCHEDULE ON SHEETS C-4 THRU C-6.**

**Alternate Standard Barn Wall Stud to Rafter Roof Detail**

- **20 GAUGE x 1-1/4" GALV. STEEL STRAP WITH (20): 0.131"x2-1/4" NAILS OR EQUAL (1) INTO STUD WALLS**

**2x6 A. PANEL-LOC PLUS ROOF PANEL by CENTRAL STATES MANUFACTURING or EQUAL**

**14 GAUGE x 1-1/4" Ti-Ii Side Siding or 14 GAUGE x 1-1/4" Ti-Ii Side Siding**

**Note:** For all fastening of framing members not noted on this sheet, refer to fastening schedule on sheets C-4 thru C-6.

--

**These plans were designed in accordance with the 2015 International Building Code, Building with A-600 Roof, Second Dyst of 160 MPH.**

---

**For All Fastening of Framing Members Not Noted on This Sheet Refer to Fastening Schedule on Sheets C-4 thru C-6.**
NOTE: FOR ALL FASTENING OF FRAMING MEMBERS NOT NOTED ON THIS SHEET REFER TO FASTENING SCHEDULE ON SHEETS C-4 THRU C-6

16 GAUGE GALVANIZED STEEL WITH (14) TOTAL (1) EACH END 0.188" x 1/2" NAILS OR EQUAL INTO RAFTERS AND STUD WALLS
SIMPSON MTS12 OR EQUAL TWIST STRAP

NOTE: USED ON EVERY WALL STUD TO CONNECT WALL TO ROOF

20 GAUGE x 1-1/4" GALV. STEEL STRAP WITH (20) 0.188" x 2-1/4" NAILS OR EQUAL (13) INTO RAFTERS AND (1) INTO STUD WALL

THOMAS A. DIXON, P.E.
DIXON ENGINEERING, INC.
STRUCTURAL ENGINEERING AND INSPECTION - 2015 IBC

COOK PORTABLE WAREHouses
STANDARD BARN SHED
100 DOUGLAS STREET
VALDOSTA, GA 31601
PHONE: 1-229-248-8005

这些计划是根据2015年国际建筑规范设计的，适用于每160英里每小时的二等风速。
(1) - 2x S.P.F. #2 with 1/2" plywood filler
See detail 2/A-13

(3) 0.131" x 3" 6D nails
Each end of header

2x4 S.P.F. #2
Laid flat

WINDOW HEADER AND SILL

A-13

NOTE:
FOR ALL FASTENING OF FRAMING MEMBERS NOT NOTED ON THIS SHEET REFER TO FASTENING SCHEDULE ON SHEETS C-4 THROUGH C-6

(1) - 2x S.P.F. #2 with 1/2" plywood filler
See detail 2/A-13

(2) - 2x S.P.F. #2 with 1/2" plywood filler
See detail 3/A-11

3'-0" DOOR HEADER

(6) 0.131" 4" x 3-1/2"
Gunnails each end of header

HEADER WITH STRAP

A-12

20 GA. STRAP TO KEEP UNDER DOOR HEADER TO OTHER SIDE
OF CRIPPLE STUD

(5) 0.131" 4" x 2-1/4"
Gunnails on each side

2x4 CRIPPLE STUD

HEADER WITH STRAP

A-12

(2) - 2x S.P.F. #2 with 1/2" plywood filler
See detail 3/A-11

3'-0" DOOR HEADER

(6) 0.131" 3'-1/2"
6D nails each end of header

CRIPPLE STUD STRAP

A-12

2x4 SP.F. flat

NOTE:
THESE PLANS WERE DESIGNED IN ACCORDANCE WITH THE 2015 INTERNATIONAL BUILDING CODE, BUILDING PARTS, SECOND EDITION OF 160 MPH.
ANCHORING GENERAL NOTES

1. THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING ALL EXISTING CONDITIONS.

2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING ARCHITECTURAL, STRUCTURAL, MECHANICAL, AND ELECTRICAL DETAIL AND DIMENSIONS. ANY DISCREPANCIES BETWEEN SUCH DETAILS AND DIMENSIONS SHALL BE REPORTED TO THE ENGINEER PRIOR TO PROCEEDING WITH THE WORK.

3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ERECTION PROCEDURE AND SEQUENCE TO INSURE THE INTEGRITY OF THE BUILDING AND ITS COMPONENT PARTS DURING CONSTRUCTION.

4. THESE PLANS HAVE BEEN PREPARED PER REGULATIONS OF THE 2014 FLORIDA BUILDING CODE. THE WORK OF ALL CONTRACTORS SHALL COMPLY WITH THE REQUIREMENTS SET FORTH IN THE FOREGOING CODE. NO DEVIATIONS FROM THE WORK SHOWN OR REASONABLY IMPLIED SHALL BE UNDERTAKEN WITHOUT THE ENGINEER'S WRITTEN CONSENT - A COPY OF WHICH WILL BE FILED WITH THE CONSTRUCTION OFFICIAL.

5. ANY CHANGES TO OR DEVIATIONS FROM THESE DRAWINGS SHALL NOT BE MADE WITHOUT WRITTEN CONSENT FROM THE ENGINEER.

6. THESE DRAWINGS ARE THE PROPERTY OF THE ENGINEER AND SHALL NOT BE USED WITHOUT HIS CONSENT. DRAWINGS SHALL NOT BE USED FOR ISSUE OF BUILDING PERMIT UNLESS SIGNED AND SEALED BY THE ENGINEER.


8. ALL WORK AND MATERIALS SHALL MEET THE REQUIREMENTS OF LOCAL AND STATE CODES AND THE SPECS OF THE NATIONAL BOARD OF FIRE UNDERWRITERS. CONTRACTORS SHALL CHECK AND VERIFY ALL PLAN DIMENSIONS AND CONDITIONS BEFORE PROCEEDING CONSTRUCTION. HE SHALL REPORT ANY DISCREPANCIES TO THE ENGINEER FOR CORRECTION PRIOR TO BEGINNING ANY WORK. THE DISCOVERY OF DISCREPANCIES AFTER THE BEGINNING OF WORK WILL BE EVIDENCE OF FAULTY WORK AND SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. DO NOT SCALE DRAWINGS ALL WRITTEN DIMENSIONS GOVERN.


11. ALL WORK SHALL BE COMPLETED IN ACCORDANCE WITH ALL STATE AND DEPARTMENT OF AGRICULTURE, STRUCTURAL PEST CONTROL DIVISION REGULATIONS, RULES, DEFINITIONS AND REQUIREMENTS.

12. THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING AND MAINTAINING ALL EXISTING SETBACKS, EASEMENTS, AND ANY DEED RESTRICTIONS.

13. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR FINAL CLEANUP AND SHALL INCLUDE THE SITE, AND THE BUILDING. THE ENTIRE PROJECT SHALL BE LEFT IN A NEW, CLEAN CONDITION.

14. THESE PLANS HAVE BEEN PREPARED PER REGULATIONS OF THE 2015 INTERNATIONAL BUILDING CODE, BUILDING MAXIMUMS SECOND 45 MPH.

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COOK PORTABLE WAREHOUSES
STANDARD BARN SHED
100 DOUGLAS STREET
VALDOSTA, GA 31601
PHONE: (229) 244-8005

ANCHOR GENERAL NOTES

DATE: 3/30/17
DRAWN BY: CNO
CHECKED BY: TAD
SCALE: AS NOTED
H.O. NO.: 445-076

### MUFRS 160 MPH EXP. "B"

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<th>ZONE</th>
<th>TABLE PRESSURE 1</th>
<th>ADJUSTMENT FACTOR 2</th>
<th>LOAD COMBINATION FACTOR 3</th>
<th>WORKING PRESSURE (PSF)</th>
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### MUFRS 130 MPH EXP. "B"

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### MUFRS 110 MPH EXP. "B"

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</table>

**NOTES:**
1. SEE FIGURE 28.6-1 PAGE 503-305 IN ASCE 1-10.
2. SEE FIGURE 28.6-1 PAGE 305 IN ASCE 1-10.
3. SEE SECTION 2.4.1 IN ASCE 1-10.
4. DESIGN PRESSURES SHOWN ARE BASED ON WORST CASE DESIGN CONDITIONS OF BUILDINGS FOR WIND VELOCITIES PER 2014 FBC.
### MWRS 160 MPH EXP. "C"

<table>
<thead>
<tr>
<th>ZONE</th>
<th>TABLE PRESSURE</th>
<th>ADJUSTMENT FACTOR</th>
<th>LOAD COMBINATION FACTOR</th>
<th>WORKING PRESSURE (PSF)</th>
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<tbody>
<tr>
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<td>0.60</td>
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<tr>
<td>E</td>
<td>-16.8</td>
<td>1.21</td>
<td>0.60</td>
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</tr>
<tr>
<td>F</td>
<td>-30.2</td>
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### MWRS 130 MPH EXP. "C"

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<tr>
<td>E</td>
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### MWRS 110 MPH EXP. "C"

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<th>WORKING PRESSURE (PSF)</th>
</tr>
</thead>
<tbody>
<tr>
<td>A</td>
<td>-23.6</td>
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<td>B</td>
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**NOTES:**
1. SEE FIGURE 28.6-1 PAGE 303-305 IN ASCE1-10.
2. SEE FIGURE 28.6-1 PAGE 305 IN ASCE1-10.
3. SEE SECTION 2A.1 IN ASCE1-10.
4. DESIGN PRESSURES SHOWN ARE BASED ON WORST CASE DESIGN CONDITIONS OF BUILDINGS FOR WIND VELOCITIES PER 2014 FBC.

---

**THOMAS A. DIXON, P.E.**

**COOK PORTABLE WAREHOUSES**

**EXPOSURE C WIND CHARTS**

---

**REV. BY DATE DESCRIPTION**

**SHEET 21 OF 26**
### Anchoring Schedule for Up to 110 MPH Wind Speed, Exposure "B"

<table>
<thead>
<tr>
<th>BLDG WIDTH</th>
<th>8'-0&quot;</th>
<th>10'-0&quot;</th>
<th>12'-0&quot;</th>
<th>14'-0&quot;</th>
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<th>18'-0&quot;</th>
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<tbody>
<tr>
<td>7'-11&quot;</td>
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<td>2</td>
<td>2</td>
<td>2</td>
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<td>9'-11&quot;</td>
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<tr>
<td>11'-0&quot;</td>
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<td>2</td>
<td>2</td>
<td>2</td>
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<td>2</td>
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### Anchoring Schedule for 111 to 130 MPH Wind Speed, Exposure "B"

<table>
<thead>
<tr>
<th>BLDG WIDTH</th>
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<th>12'-0&quot;</th>
<th>14'-0&quot;</th>
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<th>18'-0&quot;</th>
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<th>24'-0&quot;</th>
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<tbody>
<tr>
<td>7'-11&quot;</td>
<td>2</td>
<td>2</td>
<td>2</td>
<td>2</td>
<td>2</td>
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<tr>
<td>11'-0&quot;</td>
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### Anchoring Schedule for 131 to 160 MPH Wind Speed, Exposure "B"

<table>
<thead>
<tr>
<th>BLDG WIDTH</th>
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<th>12'-0&quot;</th>
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<th>22'-0&quot;</th>
<th>24'-0&quot;</th>
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</table>

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**Scale:** As Noted

**These plans were designed in accordance with the 2015 International Building Code, Building for B Zones Second Sust of 160 MPH.**
### Schedules for Anchors Placed Vertical Into Ground

#### Anchoring Schedule for Up to 110 MPH Wind Speed, Exposure "C"

<table>
<thead>
<tr>
<th>BLDG WIDTH</th>
<th>8'-0&quot;</th>
<th>10'-0&quot;</th>
<th>12'-0&quot;</th>
<th>14'-0&quot;</th>
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#### Anchoring Schedule for 111 to 130 MPH Wind Speed, Exposure "C"

<table>
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<th>BLDG WIDTH</th>
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<th>12'-0&quot;</th>
<th>14'-0&quot;</th>
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<th>24'-0&quot;</th>
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<th>30'-0&quot;</th>
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#### Anchoring Schedule for 131 to 160 MPH Wind Speed, Exposure "C"

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<tr>
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These plans were designed in accordance with the 2015 International Building Code, Building, for B and C zones, Exposure G of 160 MPH.
### Ground Anchor Schedule

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<th>Part #</th>
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<th>Soil Class</th>
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<td>48&quot; x 5/8&quot; Rod with (1) 6&quot; Helix</td>
<td>4A</td>
</tr>
<tr>
<td>M12H3/4</td>
<td>54085 / 54044</td>
<td>48&quot; x 3/4&quot; Rod with (1) 6&quot; Helix</td>
<td>4A</td>
</tr>
<tr>
<td>M4423/4</td>
<td>54128</td>
<td>42&quot; x 3/4&quot; Rod with (2) 4&quot; Helix</td>
<td>4A</td>
</tr>
<tr>
<td>M446/4</td>
<td>54086</td>
<td>48&quot; x 3/4&quot; Rod with (2) 4&quot; Helix</td>
<td>4A</td>
</tr>
<tr>
<td>M12H6/8</td>
<td>541250</td>
<td>36&quot; x 3/4&quot; Rod with (1) 4&quot; Helix, and (1) 6&quot; Helix</td>
<td>4A</td>
</tr>
<tr>
<td>N/A</td>
<td>54065</td>
<td>Eye Anchor - 48&quot; x 5/8&quot; with (1) 6&quot; Helix</td>
<td>4A</td>
</tr>
<tr>
<td>N/A</td>
<td>54045</td>
<td>Eye Anchor - 48&quot; x 3/4&quot; with (1) 6&quot; Helix</td>
<td>4A</td>
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<tr>
<td>M601</td>
<td>54099</td>
<td>60&quot; x 3/4&quot; with (1) 7&quot; Helix</td>
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<td>54040</td>
<td>Eye Anchor - 60&quot; x 3/4&quot; with (1) 8&quot; Helix</td>
<td>4B</td>
</tr>
</tbody>
</table>

**Note:**
1. All approved anchors listed above are manufactured by Tie Down Engineering.
2. The contractor may use an approved equivalent with approval from the EOR.

---

These plans were designed in accordance with the 2015 International Building Code, Building for a Share, Second Edition of 160 MPH.
NOTE: SHIMS MAY BE PLACED UNDER SKIDS IN ORDER TO LEVEL THE BUILDING.

FINISH GRADE

FINISH GRADE

TIE DOWN ENGINEERING DOUBLE TENSION PART NUMBER 34000.

FINISH GRADE

FINISH GRADE

BUILDING SKID REFER TO BUILDING PLANS FOR SIZE, LENGTH, AND LOCATION

TO SKID

STABILIZER PLATE

STABILIZER PLATE

QUICK-SET

QUICK-SET

STABILIZER PLATE

STABILIZER PLATE

OPTIONAL, 16" x 16" x 4" SOLID CAP BLOCKS REFER TO SCHEDULE SHEET F-8 OR WOOD SHIMS TO LEVEL OFF BUILDING. USE UP TO 3 BLOCKS TO LEVEL SKIDS ON GRADE & CONC. PADS VERTICAL ANCHOR UN-LEVEL GROUND

VERTICAL ANCHOR

VERTICAL ANCHOR

VERTICAL ANCHOR

ALL SKIDS ON GRADE VERTICAL ANCHOR SECTION

VERTICAL ANCHOR

VERTICAL ANCHOR

PORTABLE WAREHOUSES

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PORTABLE WAREHOSES
### 16" x 8" x 4" Pad Schedule for All Wind Speeds, Exposures, and 40 Psf Floor Load

#### Number of Pads Required by Building Length Under Each Skid

<table>
<thead>
<tr>
<th>Bldg Width</th>
<th>8'-0&quot;</th>
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<th>12'-0&quot;</th>
<th>14'-0&quot;</th>
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<th>24'-0&quot;</th>
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### Blocking Plan

**Example Drawing is 20'-0" in Length**

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**Note:**
1. Concrete pads are optional.
2. Dimensions shown are nominal.
3. Anchors are required min. (4) per building, (1) at each corner shearwall (SHE).  
4. Refer to schedules on Sheet F-4 for anchor spacing and schedules on this sheet for optional pad location.
5. Space optional pads equally.

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**Optional Pad Details**

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**These Plans Were Designed in Accordance with the 2015 International Building Code, Building, and PSE with a Design Wind Speed of 160 MPH.**

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**Area for Approval Stamps**

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